

12d Model

Civil and Surveying Software

Drainage Analysis Module

Ku & Kw Calculation

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This document describes the procedures for calculating Ku and Kw coefficients, as implemented in the *Drainage Analysis* module of *12d Model*.

For stormwater pits, maintenance holes and culvert inlets: Ku coefficients apply to pressure head (Hp) losses.

For stormwater pits and maintenance holes: Kw coefficients apply to water surface elevation (WSE) losses.

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Introduction

For the design of piped stormwater systems, the loss (or gain) in pressure head (Δ Hp) *through a pit*¹, is typically assumed proportional to the velocity head at the entrance of the downstream pipe. Likewise proportional, but sometimes of different magnitude, is the corresponding change in effective water surface elevation (Δ WSE) *between the pit and the downstream pipe*. However simple this may seem, the two coefficients of proportionality (denoted by Ku for pressure head changes, and Kw for WSE changes) are generally dependent on so many different factors, that their adequate estimation still relies largely on the results of empirical study. Perhaps the most thorough sources emanating from such study, are the so called "Missouri Charts" (Sangster *et al*, 1958) and "Hare Charts" (Hare, 1981). The *Australian Rainfall and Runoff* (ARR, 1987), suggests the use of these sets of charts, in preference to any other method.

As originally published, the charts are highly complex, varied in presentation, and somewhat open to interpretation – reflective of the chaotic nature of flow through pits. A good deal of judgement is required in selecting the appropriate chart to use for a particular pit configuration, and in most cases, iterative calculations are required. For large stormwater networks, this typically leads to a huge time-cost for the designers, or alternatively, to an overly-conservative design approach. To overcome such problems, several semi-analytical methods have been proposed, with an aim to replace the dependence on charts. These range from the relatively simple methods suggested by Argue (1986), Hare *et al* (1990)², and Mills *et al* (1998), to the more accurate methods (which are arguably as complex as using the charts manually) suggested by Parsell (1992), and Stein *et al* (1999). A summary paper (O'Loughlin *et al*, 2002) reviewed the latter four of these methods, and concluded that none matched acceptably well, across the full range of pit configurations covered by the charts, and that more work was required to develop a practical method suitable for implementation with a computer.

12d Model adopts a method that is purely numerical, rather than semi-analytical. It is based on the fact that the majority of the chart data (i.e. the charts for pits with no more than a single upstream pipe) offer a range suitable for consideration in a continuous sense. This fact has allowed the chart data to be re-arranged and combined into a single database of Ku and Kw values – one that both spreads evenly across the full range of the charts used, and is convenient for computation. The resultant database is used to calculate values which match the charts *perfectly*, for those pits which coincide with one of the discrete chart configurations; otherwise, values are calculated which indicate a *linear transition* between particular charts. The Ku and Kw calculations are *all* based on a robust, programmed sequence of one-dimensional and/or three-dimensional interpolations within the database.

The method may be thought of as a particular, holistic way of interpreting the chart data. It has been developed with an aim to minimise user input, and increase the overall efficiency of the design process. Individual charts *do not* need to be nominated at each pit, and horizontal and vertical misalignment of pipes (a key factor affecting Ku and Kw) is considered with minimal interaction. The method is most reliable for pits with either no upstream pipe, or one upstream pipe. For pits with two or more upstream pipes (for which very few discrete charts exist) a single equivalent upstream pipe is determined, yielding results which compare adequately with the limited chart data available.

For culvert inlets, *12d Model* adopts a different method, based on data published by the *U.S. Federal Highway Administration* (HDS5, 2005), to calculate Ku values for culverts flowing under inlet or outlet control. This method is detailed on page 19, and covers a wide range of different culvert configurations.

¹ The term "pit" is extended in meaning here, to include *maintenance holes* (where no grate inlet flow from above is possible).

² Sometimes referred to as the "Hare Equations". These equations differ from the "Hare Charts", in that they offer only an approximation to the *preferred configuration* charts (see configurations: G1, T1, T2, T4, T8, T10).

Ku & Kw Calculations in 12d Model

		12d	Model Pit C	Configurat	ions						
			GRAT	E PITS							
		θg =	= 0°	θg	= 90°	-					
	* Ku = Kw	Q _g G	G1 [*] Q₀	Qg	[▲] Q ₀ G2 [*]						
THROUGH PITS											
$\Theta_{u} = 0^{\circ}$	θ _u =	22.5°	θ _u =	45°	θ _u =	67.5°	$\theta_u = 90^\circ$				
Qu T1* Qo Qg		2 [*] 22.5° ⁺		$\begin{array}{c} Q_{0} \\ 4 \\ Q_{g} \\ Q_{$		² u 8 Qg 67.5° 9 Qg					

 Chart Source 1:
 QUDM (1994).

 Chart Source 2:
 ACTDS (2003).

In both source documents: Chart for Grate Pits from Sangster *et al* (1958). Charts for Through Pits from Hare (1981).

12d Pit Config	G1	G2	T1	T2	Т3	T4	T5	T6	T7	T8	Т9	T10
QUDM Ku Chart #	32	32	33	34	35	37	37	38	40	42	44	46
QUDM Kw Chart #	32	32	33	34	36	37	37	39	41	43	45	47
ACTDS Ku Chart #	1	1	2	13	14	10	9	16	18	20	22	7
ACTDS Kw Chart #	1	1	2	13	15	10	9	17	19	21	23	8
ACTDS Pit Type #	1	2	3	11	12	8	7	13	14	15	16	6

12d Controls	12d Settings	Remarks					
Ku method	Direct	Ku & Kw specified directly by user.					
	Ku,Kw via Charts	Ku & Kw via chart interpolation.					
	Ku,Kw>0 via Charts	Prevents -ve Ku or Kw from charts.					
	33 culvert methods	See Culvert Inlets on page 19.					
Ku config	Preferred	Gives lowest Ku & Kw (on average).					
	Good	:					
	Fair	:					
	Poor	Gives highest Ku & Kw (on average).					

Independent Chart Variables :

 $\begin{array}{l} \theta g = angle \ between \ grate \ flow \ line \ and \ d/s \ pipe \\ \theta u = angle \ between \ equivalent \ u/s \ pipe \ and \ d/s \ pipe \\ Qg/Qo = \ equivalent \ grate \ flow \ ratio \\ Du/Do = \ equivalent \ pipe \ diameter \ ratio \\ S/Do = \ submergence \ ratio \end{array}$

Procedural Steps :

1A) Grate Pit chart is used for pits where Qg/Qo = 1.0.

• G1 used where $\theta g \le 15^\circ$; G2 used where $\theta g > 15^\circ$.

1B) Through Pit charts are used for pits where $0.0 \le Qg/Qo \le 0.5$.

- Interpolation within a Through Pit chart is based on Qg/Qo and Du/Do.
- Interpolation between Through Pit charts is based on θu and *Ku config* : Preferred, Good, Fair, Poor.

	0°	22.5°	45°	67.5°	90°
Preferred	T1	T2	T4	T8	T10
Good	T1	T2	T5	T8	T10
Fair	T1	T3	T6	Т9	T10
Poor	T1	T3	T7	Т9	T10

1C) For pits where 0.5 < Qg/Qo < 1.0, a further (linear) interpolation based on Qg/Qo is made, between the interpolated data from steps 1A and 1B.

2) Step 1A, 1B or 1C produces a Ku and a Kw curve (both versus S/Do). Actual S/Do values are calculated to intersect the Kw curve, and thus find the applicable S/Do for the final Kw & Ku values.

How does 12d Model determine the Ku & Kw chart inputs: Qg/Qo, Du/Do, θ g, θ u? Example with grate flow and 3 u/s pipes:

Qu2->Qo

Qu3->Qo

90

30

40

590

1.867

0.044

90

54.7

Qg/Qo	equivalent g	grate flow rate	io			(Steps: 1A,	1B, 1C)	
	From	Rescaled to						
	Rational	Conserve						
	Method	Mass						
	Qrat	Qeq						
Qg	53.0	48.8						
Qu1	246.0	226.7						
Qu2	151.0	139.2						
Qu3	98.0	90.3						
Qu	495.0	456.2						
Qg + Qu	548.0	505.0						Qg/Qo = (Qg)eq / Qo
Qo	505.0	505.0			Qg/Qo =	48.8/505.0 =	0.097	0.0 <= Qg/Qo <=1.0
Du/Do	equivalent p	oipe diameter	ratio			(Steps:	1B, 1C)	
Pipe Diame	ters	Pipe Areas						
Du1	375	Au1	0.110					
Du2	300	Au2	0.071					
Du3	225	Au3	0.040					
		Au	0.221	1				Du/Do = SQR(Au/Ao)
Do	600	Ao	0.283		Du/Do =	SQR(.221/.283) =	0.884	0.6 <= Du/Do <= 1.0
θg	angle betwe	en grate flow	line and d/s	pipe		(Steps:	1A, 1C)	
Angle betwe	een setout str	ing and d/s pi	ipe		θg =		32.0	$0^{\circ} \le \theta g \le 90^{\circ}$
θu	angle betwe	en equivalent	t u/s pipe and	d d/s pipe		(Steps:	1B, 1C)	
	Horiz 0	Drop	VAF*	θ				$\theta u = \theta 1.Qu 1/Qu$
Qu1->Qo	0	20	1.547	0	θu =	(0.0) 226.7/456.2 +		$+ \theta 2.Qu 2/Qu$

*VAF = Vertical Alignment Factor = (Do - Drop) / Du For VAF < +0.25 (i.e. excessive vertical misalignment),

 θ is increased linearly to compensate, viz: +0.25 > VAF > -0.25 Horiz $\theta < \theta < 90^{\circ}$

38.3

 $+ \theta 3.Qu 3/Qu$

 $0^{\circ} \le \theta u \le 90^{\circ}$

(90.0) 139.2/456.2 +

(54.7) 90.3/456.2 =

Grate Pits (i.e. grate flow only) Example of Kw calculation (Ku = Kw) :

INPUT			Chart :	G1	G2
θg	32.0		θg	0°	90°
Do	0.300		S/Do	Kw	Kw
Vo	0.920		1.5	7.00	9.70
ILo	27.224		2.0	4.80	7.00
HGLo	27.900		2.5	3.75	4.90
		-	3.0	3.15	3.80
			4.0	2.45	2.72
			5.0	2.10	2.20
			6.0	1.90	1.98
			7.0	1.80	1.87
,					

	(G2)				
	(>15°)				
	Kw.Vo ² /2g	WSE	S	S/Do	Error
)	0.419	28.319	1.095	3.650	-2.150
)	0.302	28.202	0.978	3.261	-1.261
)	0.212	28.112	0.888	2.959	-0.459
)	0.164	28.064	0.840	2.800	0.200
2	0.117	28.017	0.793	2.645	1.355
)	0.095	27.995	0.771	2.570	2.430
5	0.086	27.986	0.762	2.538	3.462
1	0.081	27.981	0.757	2.523	4.477

G1 used where $\theta g \le 15^{\circ}$ G2 used where $\theta g > 15^{\circ}$ (as per Chart 32 of QUDM, 1994)



Through Pits (i.e. through flow and grate flow) Example of Ku & Kw calculation :























Other Chart Data

Both source documents contain other charts not used by *12d Model*, covering the following limited set of four pit configurations, each with two upstream pipes:



The charts for M3, M4, and (under certain conditions) M2, suggest *slightly* independent (i.e. *slightly* different) Ku values for each of the two upstream pipes. However, the peak flows in each pipe (determined by *12d Model* via the Rational Method) do not, in general, occur at the same moment in time, and so provide little justification to account for these slight differences. As such, *12d Model* supports only a single Ku (and a single Kw) at each pit. The Rational Method is a statistical design method with much to commend it, but it is not sophisticated enough for these particular charts, which are perhaps better suited to a method based on unsteady flow simulations.

Using the *12d Model* method (of determining a single equivalent upstream pipe), the Ku and Kw values for M1 and M2 may be estimated adequately, if a little conservatively, with the *Ku config* set to "Fair". For M3 and M4, adequate estimations are made regardless of the *Ku config* setting.

For other configurations of multiple upstream pipes – especially those where the jet of each upstream pipe projects wholly into the downstream pipe – $Ku \ config$ settings of "Preferred" or "Good", may be more appropriate.

Notes :

The "Missouri Chart" used by *12d Model* for Grate Pits, possibly suggests conservatively high Kw values at low submergence ratios, compared with the evidence suggested by some other empirical and analytical studies. However, due to the typically low velocity head in the downstream pipe, a high Kw value rarely makes a significant difference in Grate Pits.

The "Hare Charts" used by *12d Model* for Through Pits, are all based on *square* pits with sides twice the diameter of the downstream pipe. Some of the comparable "Missouri Charts" consider pits with other geometries, and typically suggest lower Ku values.

If difficulties are encountered in adequately matching Ku and Kw values to a particular chart not considered by *12d Model*, simply set the *Ku method* to "Direct", and enter the chart values manually. Alternatively, consider contacting the author (email: owen.thornton@12d.com) with information about the chart in question.

Part-full Flow and/or Extreme Submergence Ratios

Strictly speaking, the chart data are only applicable to pipes flowing full and under pressure. For pipes flowing part-full through pits, it is most common to assume reduced magnitudes of Δ Hp and Δ WSE, which immediately questions the validity of the assumption that the head changes are proportional to the (higher, part-full) velocity head. Some of the current Australian design manuals provide estimation procedures for these uncertain cases, but they can often result in *increased* magnitudes of Δ Hp and Δ WSE. In an attempt to provide a compromise, 12d Model employs a procedure to ensure the magnitudes are neither increased nor reduced. At those pits where the downstream pipe flows part-full, 12d Model determines Ku and Kw by assuming the HGL to be at the *obvert* of the pipe (with Δ Hp and Δ WSE based on the *full pipe* velocity head). Once the head changes are determined in this way, they are applied from the calculated HGL in the downstream pipe, *not* the pipe obvert.

The chart data for Grate Pits give Kw values for S/Do ranging from 1.5 to 7.0, and for Through Pits give Ku and Kw values for S/Do ranging from 1.5 to 4.0. In *12d Model*, the chart data are assumed to be applicable for all S/Do values greater than or equal to 1.0, with Ku and Kw extending horizontally beyond the chart limits, when plotted against S/Do. With the "obvert assumption" outlined above for part-full flow, it is only ever a *negative* Kw value that may *potentially* cause S/Do to be calculated less than 1.0. Since this situation is far enough outside the range of the charts to be deemed "in doubt", *12d Model* handles such instances by simply (and conservatively) increasing the Ku and Kw values, so as to give S/Do equal to 1.0.



An Example of Part-full Flow

Culvert Inlets

For a pipe or box culvert, 12d Model can calculate a Ku value to give the required loss in pressure head through the culvert entrance. The Ku value is the greater of the two values determined from consideration of the culvert under inlet and outlet control. The user need only set the Ku method to one of the 33 different methods applicable to culverts, as shown below.

12d Model	HDS5	HDS5	Inlet C	ontrol Unsul	omerged	Inlet Contro	l Submerged	Inlet Control	Outlet Control	
Ku Method	Chart No.	Chart Scale	Form	K	М	с	Y	F	Ke	
Ku - 101 Pipe Culvert Inlet - Concrete; Square Edge with Headwall	1	1	1	0.0098	2.000	0.03980	0.670	-0.5	0.5	
Ku - 102 Pipe Culvert Inlet - Concrete; Socket End with Headwall	1	2	1	0.0018	2.000	0.02920	0.740	-0.5	0.2	
Ku - 103 Pipe Culvert Inlet - Concrete; Socket End Projecting	1	3	1	0.0045	2.000	0.03170	0.690	-0.5	0.2	
Ku - 104 Pipe Culvert Inlet - CMP; Headwall	2	1	1	0.0078	2.000	0.03790	0.690	-0.5	0.5	
Ku - 105 Pipe Culvert Inlet - CMP; Mitred to Slope	2	2	1	0.0210	1.330	0.04630	0.750	+0.7	0.7	
Ku - 106 Pipe Culvert Inlet - CMP; Projecting	2	3	1	0.0340	1.500	0.05530	0.540	-0.5	0.9	
Ku - 107 Pipe Culvert Inlet - Beveled ring 45°	3	Α	1	0.0018	2.500	0.03000	0.740	-0.5	0.2	
Ku - 108 Pipe Culvert Inlet - Beveled ring 33.7°	3	В	1	0.0018	2.500	0.02430	0.830	-0.5	0.2	
Ku - 109 Pipe Culvert Inlet - Concrete; Tapered Inlet Throat	55	1	2	0.5340	0.555	0.01960	0.900	-0.5	0.2	
Ku - 110 Pipe Culvert Inlet - CMP; Tapered Inlet Throat	55	2	2	0.5190	0.640	0.02100	0.900	-0.5	0.2	
Ku - 201 Box Culvert Inlet - 30° to 70° Wingwalls	8	1	1	0.0260	1.000	0.03470	0.810	-0.5	0.4	
Ku - 202 Box Culvert Inlet - 90° Headwall or 15° Wingwalls	8	2	1	0.0610	0.750	0.04000	0.800	-0.5	0.5	
Ku - 203 Box Culvert Inlet - 0° Wingwalls (Extension of Sides)	8	3	1	0.0610	0.750	0.04230	0.820	-0.5	0.7	
Ku - 204 Box Culvert Inlet - 45° Wingwalls; d=D/24 Top Bevel	9	1	2	0.5100	0.667	0.03090	0.800	-0.5	0.2	
Ku - 205 Box Culvert Inlet - 18° to 33.7° Wingwalls; d=D/12 Top Bevel	9	2	2	0.4860	0.667	0.02490	0.830	-0.5	0.2	HW = Headwater depth above IL [m]
Ku - 206 Box Culvert Inlet - 90° Headwall; 20mm Chamfers	10	1	2	0.5150	0.667	0.03750	0.790	-0.5	0.2	IL = Invert level at culvert entrance [m]
Ku - 207 Box Culvert Inlet - 90° Headwall; 45° Bevels	10	2	2	0.4950	0.667	0.03140	0.820	-0.5	0.2	HGLo = HGL level inside culvert entrance [m
Ku - 208 Box Culvert Inlet - 90° Headwall; 33.7° Bevels	10	3	2	0.4860	0.667	0.02520	0.865	-0.5	0.2	U = SI units factor = 1.811
Ku - 209 Box Culvert Inlet - 45° Skewed Headwall; 20mm Chamfers	11	1	2	0.5450	0.667	0.04505	0.730	-0.5	0.2	Q = Design flow rate per culvert barrel [ci
Ku - 210 Box Culvert Inlet - 30° Skewed Headwall; 20mm Chamfers	11	2	2	0.5330	0.667	0.04250	0.705	-0.5	0.2	D = Height of culvert barrel [m]
Ku - 211 Box Culvert Inlet - 15° Skewed Headwall; 20mm Chamfers	11	3	2	0.5220	0.667	0.04020	0.680	-0.5	0.2	A = Cross-sectional area of culvert barrel
Ku - 212 Box Culvert Inlet - 10° to 45° Skewed Headwall; 45° Bevels	11	4	2	0.4980	0.667	0.03270	0.750	-0.5	0.2	S = Slope of culvert barrel [m/m]
Ku - 213 Box Culvert Inlet - 45° Non-offset Wingwalls; 20mm Top Chamfer	12	1	2	0.4970	0.667	0.03390	0.803	-0.5	0.2	d _c = Critical flow depth in culvert [m]
Ku - 214 Box Culvert Inlet - 18.4° Non-offset Wingwalls; 20mm Top Chamfer	12	2	2	0.4930	0.667	0.03610	0.806	-0.5	0.2	V _c = Critical flow velocity in culvert [m/s]
Ku - 215 Box Culvert Inlet - 30° Skew; 18.4° Non-offset Wingwalls; 20mm Top Chamfer	12	3	2	0.4950	0.667	0.03860	0.710	-0.5	0.2	V_f = Full pipe velocity in culvert = Q/A [m
Ku - 216 Box Culvert Inlet - 45° Offset Wingwalls; d=D/24 Top Bevel	13	1	2	0.4970	0.667	0.03020	0.835	-0.5	0.2	g = Acceleration due to gravity [m/s/s]
Ku - 217 Box Culvert Inlet - 33.7° Offset Wingwalls; d=D/12 Top Bevel	13	2	2	0.4950	0.667	0.02520	0.881	-0.5	0.2	$\mathbf{K} = \text{Coefficient}[-]$
Ku - 218 Box Culvert Inlet - 18.4° Offset Wingwalls; d=D/12 Top Bevel	13	3	2	0.4930	0.667	0.02270	0.887	-0.5	0.2	$\mathbf{M} = \text{Coefficient}[-]$
Ku - 219 Box Culvert Inlet - Tapered Inlet Throat	57	1	2	0.4750	0.667	0.01790	0.970	-0.5	0.2	c = Coefficient [-]
Ku - 220 Box Culvert Inlet - Side Tapered Inlet Throat; Less Favourable Edges	58	1	2	0.5600	0.667	0.04460	0.850	-0.5	0.2	Y = Coefficient [-]
Ku - 221 Box Culvert Inlet - Side Tapered Inlet Throat; More Favourable Edges	58	2	2	0.5600	0.667	0.03780	0.870	-0.5	0.2	F = Coefficient [-]
Ku - 222 Box Culvert Inlet - Slope Tapered Inlet Throat; Less Favourable Edges	59	1	2	0.5000	0.667	0.04460	0.650	-0.5	0.2	Ke = Energy-head loss coefficient [-]
Ku - 223 Box Culvert Inlet - Slope Tapered Inlet Throat; More Favourable Edges	59	2	2	0.5000	0.667	0.03780	0.710	-0.5	0.2	Ku = Pressure-head change coefficient [-]

HW	= Headwater depth above IL [m]
IL	= Invert level at culvert entrance [m]
HGLo	= HGL level inside culvert entrance [m]
U	= SI units factor = 1.811
Q	= Design flow rate per culvert barrel [cumecs]
D	= Height of culvert barrel [m]
Α	= Cross-sectional area of culvert barrel [m ²]
S	= Slope of culvert barrel [m/m]
d _c	= Critical flow depth in culvert [m]
Vc	= Critical flow velocity in culvert [m/s]
V_{f}	= Full pipe velocity in culvert = Q/A [m/s]
g	= Acceleration due to gravity [m/s/s]
K	= Coefficient [-]
М	= Coefficient [-]
с	= Coefficient [-]
Y	= Coefficient [-]
F	= Coefficient [-]
Ke	= Energy-head loss coefficient [-]

Unsubmerged Form 1: HW/D = $(d_c + V_c^2/2g)/D + K [U.Q/(A\sqrt{D})]^M + F.S$... U.Q/(A\sqrt{D}) < 3.5 Inlet Control: **Unsubmerged Form 2:** HW/D = **K** $[U.Q/(A\sqrt{D})]^{M}$... U.Q / $(A\sqrt{D}) < 3.5$ Submerged: HW/D = $c [U.Q/(A\sqrt{D})]^2 + Y + F.S$... U.Q / $(A\sqrt{D}) > 4.0$

 $Ku = (HW + IL - HGLo) \cdot 2g / V_f^2$

Outlet Control:

Ku = Ke + 1.0

Notes:

- 1) A transition zone exists between the unsubmerged and submerged cases under inlet control. Results are obtained within this zone, via linear interpolation between the results at the limit of each case.
- 2) Technically, HW applies to the Total Energy Line (TEL), and not the Hydraulic Grade Line (HGL). However, the difference between the TEL and HGL in the upstream channel (i.e. the velocity head in the channel) is typically small, and is most commonly assumed to be negligible.
- 3) Culvert slope, S, is not squared in the these formulae ... the source document (p 192, eqn 26 & 28) refers to "NOTE 2", not "S to the power of 2".

Data Source: HDS5 (2005).

Design Checks

Spreadsheet reports may be generated quickly and easily, to allow checking and auditing of the Ku and Kw values calculated by *12d Model*.

From the *Import/Export* panel of the *Drainage Network Editor*, simply *Export* to the *Spreadsheet clipboard*, using the *Customised list file* supplied in the installed *Library* folder (as shown below), and paste the results into your spreadsheet.

Drainage Network Editor: Import/Export									
Drainage model	drainage								
I/O format	Spreadsheet clipboard	_							
I/O file name	clipboard.txt								
 Export Export catchment det Export bypass flow de Export pipe inverts ar Spreadsheet option Export all inction 	ails stails nd sizes s								
Preset output	Customised list file								
List file name	\$LIB\drainage_ku_calc_check.txl								

C Import Hold obverts on import	Г
Generate plan results Generate plan PPF Model for plan results	gn.drainplanppf
Full clean of model beforehand	
Generate long-section results Drainage long-section PPF	design.drainppf
Model stem for long-section results Clean model(s) beforehand	Jrainage LS plot
Run Back to Editor	Help

Standard "Customised list file": \$LIB\drainage_ku_calc_check.txt

header													
Pit	Ku	Kw	V'head	P'head Loss	WSE Loss	Ku	Ku	Qg/Qo	Grate Flow	Pipe Flow	Du/Do	S/Do	Chart(s)
header													
Name				(Ku.V'head)	(Kw.V'head)	Method	Config	Ratio	Deflection	Deflection	Ratio	Ratio	Used
header													
(-)	(-)	(-)	(m)	(m)	(m)	(-)	(-)	(-)	(deg)	(deg)	(-)	(-)	(-)
pit data													
pit name													
calculated	ku												
calculated kw													
pipe data													
calculated	veloci	ty head											
pit data													
calculated	pit pre	essure h	ead loss										
calculated	pit ws	e loss											
calculated	ku me	thod											
calculated	ku cor	nfig											
calculated	ku gra	te flow	ratio										
calculated	ku gra	te flow	angle										
calculated	ku pip	e flow	angle										
calculated	ku dia	meter r	atio										
calculated	ku sub	mergei	nce ratio										
calculated	ku cha	urt											

Sample Ku & Kw Design Check Report :

Pit	Ku	Kw	V'head	P'head Loss	WSE Loss	Ku	Ku	Qg/Qo	Grate Flow	Pipe Flow	Du/Do	S/Do	Chart(s)
Name				(Ku.V'head)	(Kw.V'head)	Method	Config	Ratio	Deflection	Deflection	Ratio	Ratio	Used
(-)	(-)	(-)	(m)	(m)	(m)	(-)	(-)	(-)	(deg)	(deg)	(-)	(-)	(-)
1.6H	1.11		0.21	0.23	0.23	Inlet Headwall							Inlet Control
2.1S	1.83		0.07	0.13	0.13	Ku,Kw via Charts	Preferred	0.50		27.7	0.72	1.56	T2/T4
2.2S	7.37		0.06	0.48	0.48	Ku,Kw via Charts		1.00	90.0			1.93	G2
3.1G	0.72		0.57	0.41	0.41	Ku,Kw via Charts	Preferred	0.08		30.2	1.00	1.88	T2/T4
3.2G	0.42		0.49	0.21	0.21	Ku,Kw via Charts	Preferred	0.14		0.0	0.89	1.91	T1
3.4M	0.82	0.83	0.36	0.29	0.30	Ku,Kw via Charts	Good	0.00		48.3	1.00	2.74	T5/T8
3.8M	0.55		0.19	0.11	0.11	Ku,Kw via Charts	Good	0.00		35.5	1.00	2.83	T2/T5
3.9S	1.07		0.17	0.18	0.18	Ku,Kw via Charts	Preferred	0.26		29.5	1.00	3.48	T2/T4
3.10G	0.34		0.50	0.17	0.17	Ku,Kw via Charts	Good	0.00		25.9	1.00	4.79	T2/T5
3.11M	1.73	1.88	0.23	0.40	0.44	Ku,Kw via Charts	Fair	0.00		48.7	1.00	2.46	T6/T9
3.12G	0.83		0.24	0.20	0.20	Ku,Kw via Charts	Good	0.23		11.3	1.00	3.88	T1/T2
3.13G	1.59	1.63	0.17	0.27	0.28	Ku,Kw via Charts	Preferred	0.53	37.0	63.5	1.00	3.95	G2/T4/T8
3.14G	5.86		0.04	0.23	0.23	Ku,Kw via Charts		1.00	0.0			1.76	G1

References

*ACTDS (2003) "ACT Design Standards for Urban Infrastructure",

DS01 - Stormwater, Appendix A, *ACT Dept. of Territory and Municipal Services, Canberra*, (available from: www.tams.act.gov.au/work/design_standards_for_urban_infrastructure).

Argue J.R. (1986) "Storm drainage design in small urban catchments: a handbook for Australian Practice", Special Report No. 34, *Australian Road Research Board, Vermont South, Victoria.*

ARR (1987) "Australian Rainfall and Runoff : A Guide to Flood Estimation", Vol. 1, *Instn. Engrs. Aust.*

- Hare C.M. (1981) "Energy Losses in Pipe Systems", Advances in Urban Drainage Design, *Insearch Ltd, NSW Institute of Technology*.
- Hare C.M., O'Loughlin G.G. & Saul A.J. (1990) "Hydraulic Losses at Manholes in Piped Drainage Systems", Proc. International Symposium on Urban Planning and Stormwater Management, Kuala Lumpur.

HDS5 (2005) "Hydraulic Design of Highway Culverts",
 2nd Edition, May 2005, (Report FHWA-NHI-01-020 HDS No. 5),
 U.S. Federal Highway Administration, Virginia.
 (available from: www.fhwa.dot.gov/engineering/hydraulics/library arc.cfm?pub number=7).

- Mills S.J. & O'Loughlin G. (1998) "Workshop on urban piped drainage systems", Swinburne University of Technology and University of Technology, Sydney, (also earlier editions from 1982).
- O'Loughlin G. & Stack B. (2002) "Algorithms for Pit Pressure Changes and Head Losses in Stormwater Drainage Systems", Proc. 9th International Conference On Urban Drainage, Portland, Oregon.
- Parsell R. (1992) "Generalised Equations for Estimating Pressure Change Coefficients at Stormwater Pit Junctions", Proc. International Conference on Urban Stormwater Management, Instn. Engrs. Aust.

QUDM (1994) "Queensland Urban Drainage Manual",

Vol. 2, Queensland Govt. Dept. of NRM&W, Brisbane.

- Sangster W.M., Wood H.W., Smerdon E.T. & Bossy H.G. (1958) "Pressure changes at storm drain junctions", Engineering Series Bulletin No. 41, *Engineering Experiment Station, University of Missouri*.
- Stein S.M., Dou X., Umbrell E.R. & Jones J.S. (1999) "Storm Sewer Junction Hydraulics and Sediment Transport", U.S. Federal Highway Administration, Virginia.

*ACTDS (2003) - Errata in Appendix A

Chart 5

a) Re the "Ku(bar) vs Dl/Do" graph: the y-axis range of Ku(bar) should be 1.3 to 2.5 (not 0.6 to1.8).

Chart 8

a) Re the "S/Do=2.5" graph: the "Qg/Qo=0.5" arrow label points to the wrong line.

- b) Re the "S/Do=3.0" graph: the y-axis range of Kw should be 1.8 to 2.6 (not 2.6 to 3.4).
- c) Re the "S/Do=4.0" graph: the y-axis range of Kw should be 1.6 to 2.4 (not 2.4 to 3.2).

Chart 12

a) Re the "H" graph (on left): missing "Qhv/Qo" arrow label. b) Re the "L" graph (on right): missing "Qlv/Qo" arrow label.

Chart 16

- a) Re the "S/Do=1.5" graph: the "Qg/Qo" brace labels should have 0.0 and 0.5 swapped around.
- b) Re the "S/Do=2.0" graph: the "Qg/Qo" brace labels should have 0.0 and 0.5 swapped around.
- c) Re the "S/Do=2.5" graph: the "Qg/Qo" brace labels should have 0.0 and 0.5 swapped around.
- d) Re the "S/Do=3.0" graph: the lower arrow label should be "Qg/Qo=0.5" (not "Qg/Qo=0.0").

Chart 20

a) Re the "S/Do=3.0" graph: the y-axis range of Ku should be 0.8 to 1.8 (not 1.0 to 2.0).